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# SEISMIC RETROFITTING OF THE PARLIAMENT BUILDING OF REPUBLIC OF NORTH MACEDONIA - NECESSITY, SOLUTIONS AND CONSTRUCTION

*“Seismic assessment and retrofitting of masonry and preserved structures”  
Athens, 13 September 2023*

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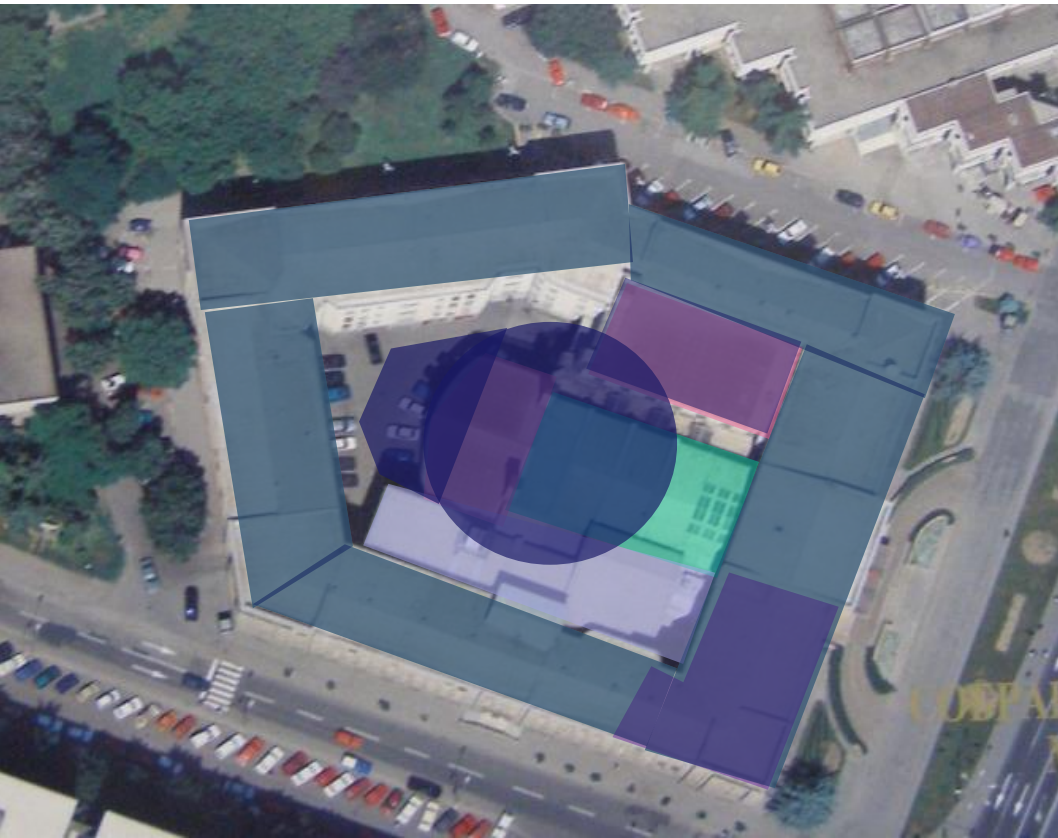


**Project for enlargement, building of another storey and adaptation of the Parliament building**



- ? definition of possibilities and conditions**
- ? estimation of existing stability  
(complex, specific, responsible task)**
- ? strengthening of the main structural system**

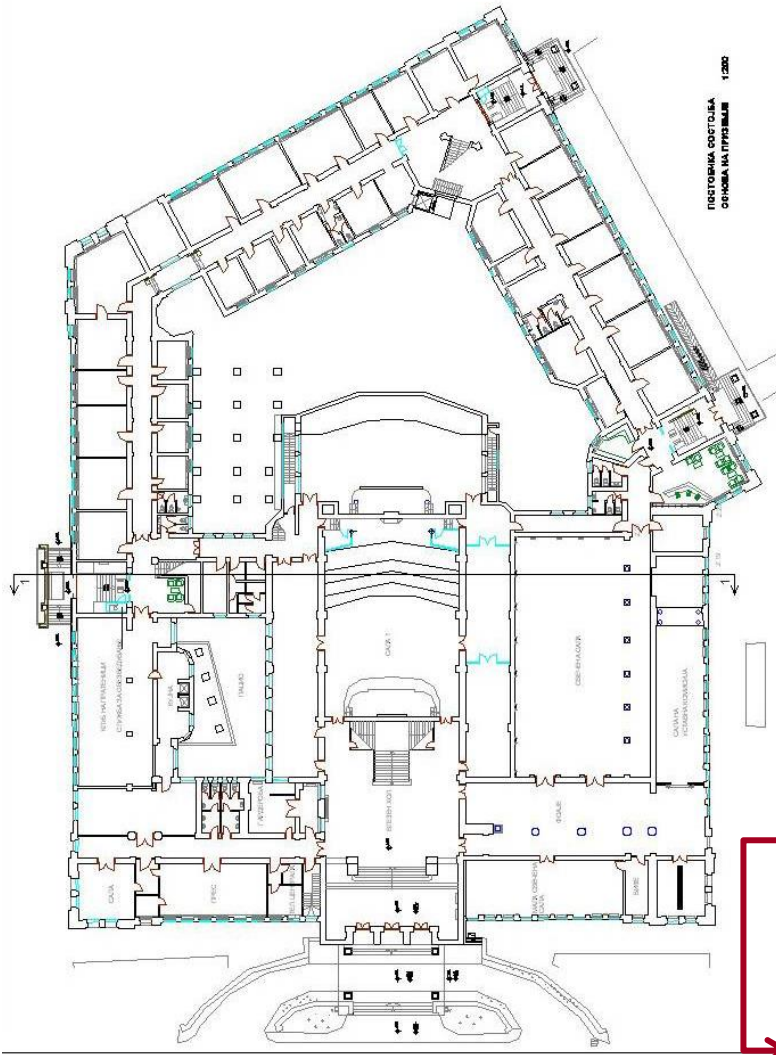
# Knowledge from technical, written and photo documentation



- 1996** reconstruction – north-east angle  
(implementation of RC elements)
- 2005** Project for enlargement, building of another story over pentagon, adaptation (~3600 m<sup>2</sup>)

- 1936-1939** Original structure-pentagon  
(brick masonry walls in both directions)
- 1936-1939** - Main Hall 1  
(reinforced concrete frame structure)
- 1954** - first enlargement - Hall 2  
(reinforced concrete frame structure)
- 1954** - first adaptation - Crystal Hall  
(replacement of existing masonry with RC columns and still beams)
- 1963** - post earthquake repair  
(injection, rebuilding of collapsed part  
grouting of partition walls, pre-stressing of walls, consolidation of parapets)
- 1964-1965** – second adaptation - Halls  
(strengthening by RC belt courses)
- 1967** second enlargement – Halls 3,4  
(reinforced concrete frame structure)

# Knowledge from technical, written and photo documentation



## Main structural system of pentagon

- ✓ according to *CONSTRUCTOR -Maribor and ZRMK Ljubljana (1964)*

*“...massive brick walls combined with concrete belt courses and columns...”*

*“...approximate analysis of repaired building shows that global safety coefficient is 2, (for  $K_s=0.12$ )....”*

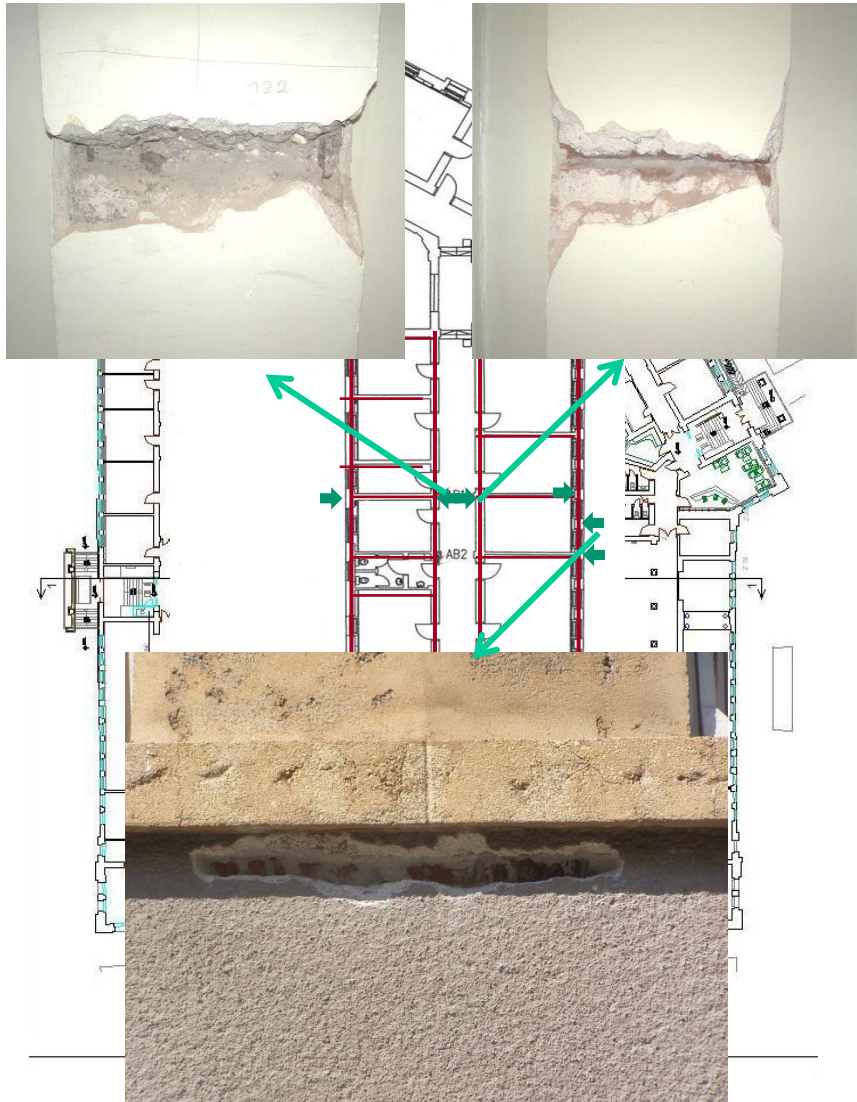
- ✓ according to *Technical conditions for building of another story (1996)*

*“...massive brick walls framed with RC horizontal and vertical belt courses, system known as confined masonry....”*

*“...there is a possibility for building another story with low live loads, but after additional investigation....”*

*Project for enlargement, building of another story & adaptation (2005)*

# Performed Technical Investigation



## **Main reason:**

- Precise definition of the main structural system since there is crucial difference between “plain” and “confined” masonry;
- Identification of RC vertical belt courses and compare with the code requirements for “confined masonry”;

## **Methodology:**

- Selection of southwest unit as representative structural unit;
- Detail nondestructive testing of southwest unit using Proceq Profometer 5;
- Confirmation of findings by minimum number of destructive testing.

# Conclusions for structural system of the pentagon shaped building

## ■ Identified RC vertical elements in selected structural unit

Intensity, placement and quality of built-in materials are not sufficient for “confined masonry” according to the requirements in valid technical regulations:

- all corners and intersection of walls
- all free ends of walls with  $d > 19\text{cm}$
- at max distance of 5m

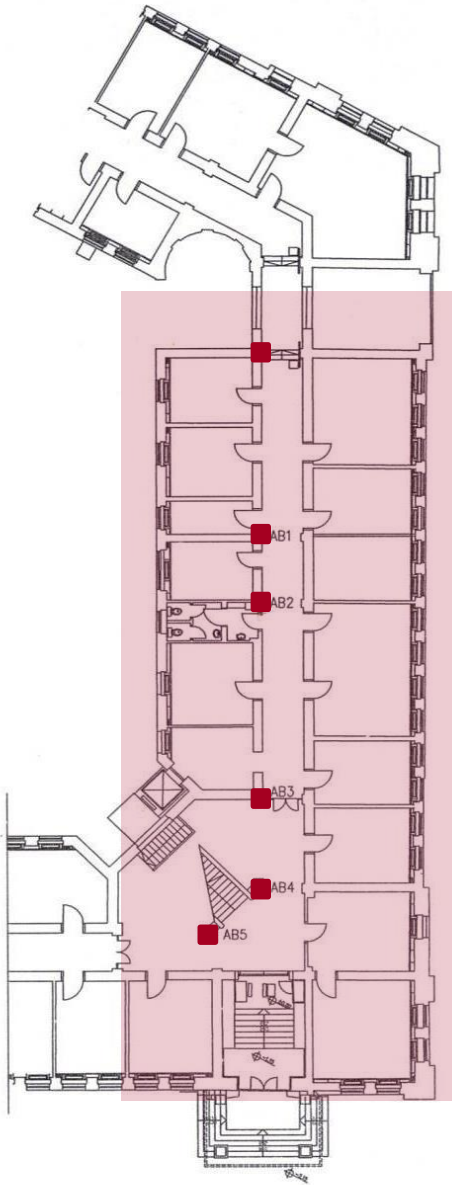
Global seismic safety:

- ~2 according *1965 Code* for  $K_s = 0.12$
- less than 1 according *1981 Code* for  $K_s = 0.30$

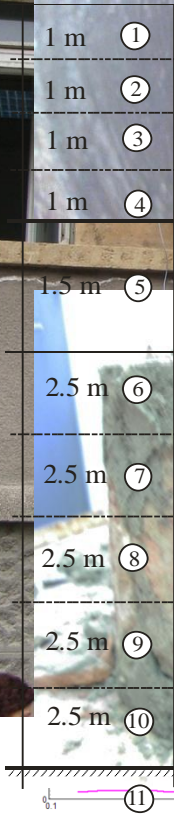
Existing structural system has unknown bearing and deformability capacity

Conditions for building of another story:

1. Detail analysis of seismic stability
2. Structural strengthening (allowed number of stories)
3. Dynamic analysis for expected earthquakes (as structure of first category)
4. Additional necessary investigation



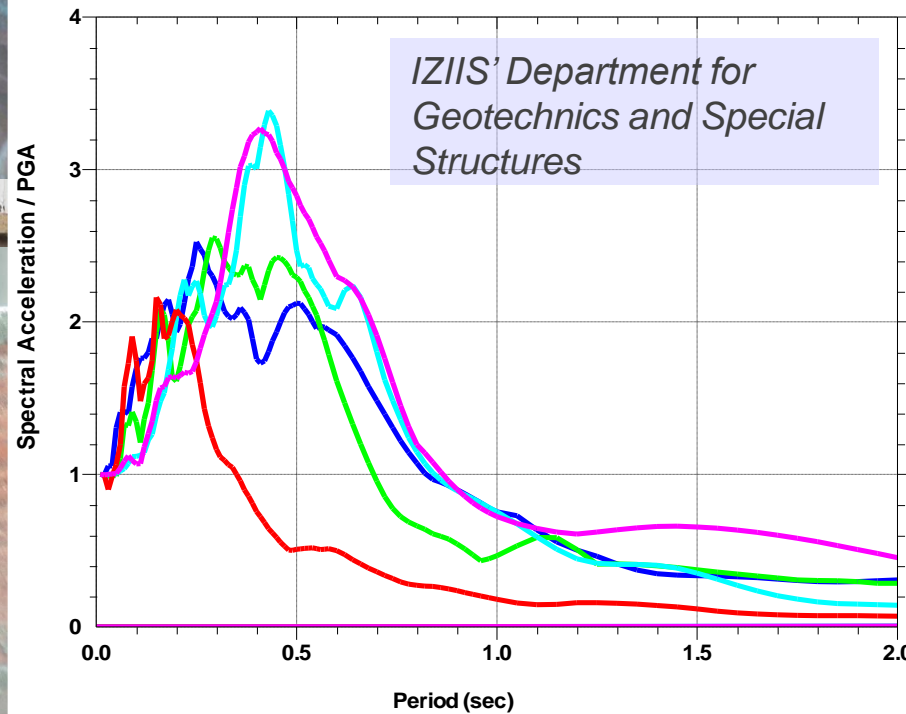
# Seismic Strengthening of Existing Structure



## Performed additional investigation:

- Definition of geotechnical and geodynamical models of the site;
- Definition of seismic parameters for evaluation of seismic stability of existing structure;

Soil Profile No. 1 - Layer 5



- Normalized PSA for 10% damping - ELCENTRO
- Normalized PSA for 10% damping - ALBATROS
- Normalized PSA for 10% damping - PETROVAC
- Normalized PSA for 10% damping - Robic
- Normalized PSA for 10% damping - PARKFIELD

characteristics;

it and in situ

## Investigation:

Additional story,

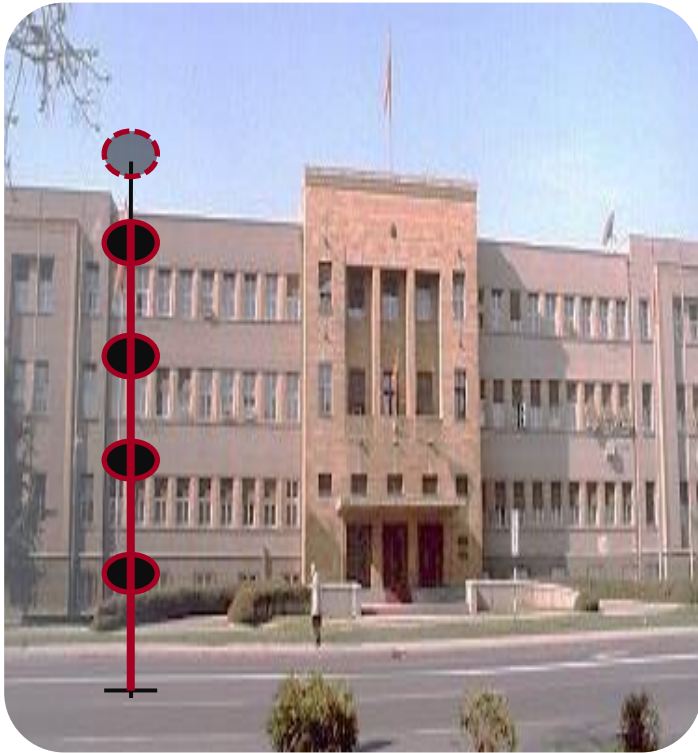
to its limit

stability capacity defined

Normalized response spectra analysis of variant solutions

Analysis of variant solutions from the aspect of stability, economy and possibility for construction

# Seismic Strengthening of Existing Structure



1. Existing
2. With additional story
3. Strengthened

## Definition of safety criteria:

Level I – elastic behavior, beginning of nonlinearity  
 $\mu < 1.5$ , for  $t_p = 100$  years,  $a_{\max} = 0.27g$

Level II – nonlinear behavior  
 $1.5 < \mu < 2.5$ , for  $t_p = 475$  years,  $a_{\max} = 0.38g$

Level III – deep nonlinearity, but non-disturbed stability  
 $2.5 < \mu < 3.5$ , for  $t_p = 950$  years,  $a_{\max} = 0.42g$

## Bearing and Deformability Capacity:

Bearing Capacity = ultimate story transversal force  $Q_u$ , which compared with the equivalent seismic force gives the safety factor against failure,  $F_u = Q_u / S_i$

Deformability Capacity = max ductility as relationship between maximal deformation and deformation at yield point,  $\mu_{\max} = \delta_{\max} / \delta_y$

## Nonlinear Dynamic Analysis:

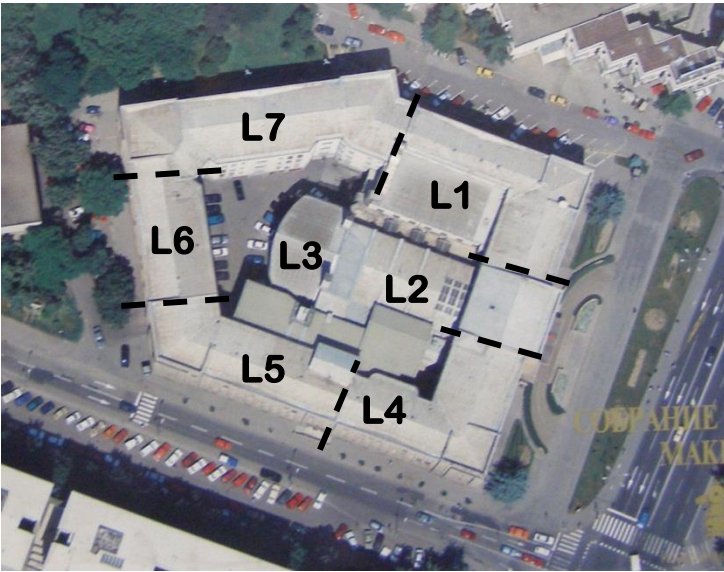
Masses - concentrated at floor levels

Hysteretic models - by summing of elasto-plastic characteristics of separate walls



# 1. Analysis of Existing State

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## **INPUT:**

Shear base  $K = 0.30$  (30% G)

## **Masonry:**

$E = 800000$  kPa;

$f_c = 1200-2200$  kPa;  $f_t = 120-220$  kPa;

$\gamma_{\text{brick}} = 18.5$  kN/m<sup>3</sup>

## **Concrete:**

MB 16-23 MPa, GA 240/360

## **OUTPUT:**

Bearing Capacity for L1-L7: 6-18% G, required one - 30%G

Ductility Capacity for L1-L7: 1.4 – 2.1, required one - up to 3.5

**The necessity for structural strengthening is analytically approved!**

## 2. Analysis of Existing State with additional story

**3 types of modeling:** **SW** - separate walls - as merittory  
**AW** - adjoining walls - as control  
**P** - pentagon - without dilatations

Structural Unit	Required bearing capacity (% of weight)	Bearing capacity (% of weight)		Required ductility (maximal)		Ductility capacity (maximal)	
		x-x	y-y	x-x	y-y	x-x	y-y
<b>Existing state – separate walls SW</b>							
L1	30	14.95	14.39	3.72	2.93	2.05	1.42
L2		16.75	22.18	3.58	4.00	2.42	2.30
L4		12.34	10.34	2.90	2.77	1.71	1.92
L5		11.54	12.50	3.33	2.81	1.63	1.72
L6		18.68	7.09	2.21	3.60	1.53	1.97
L7		13.08	13.13	3.10	3.20	1.71	1.65
<b>Existing state with adjoining walls, AW</b>							
L1	30	20.20	16.10	2.10	2.57	1.42	1.56
L2		17.70	27.30	2.53	4.30	2.37	3.60
L4		18.00	15.80	2.10	2.10	1.52	1.92
L5		20.40	13.10	1.75	2.90	1.46	1.48
L6		30.80	6.70	1.77	3.00	1.27	1.24
L7		21.10	17.80	1.99	1.50	1.55	1.47
<b>Existing state without dilatation P</b>							
P	30	17.8	17.00	2.50	2.55	1.62	1.63

# Technical Solution for Structural Strengthening

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## **based on:**

- required additional strength and deformability (demand - existing)
- requirements in technical regulation
- minimum intervention for optimal results

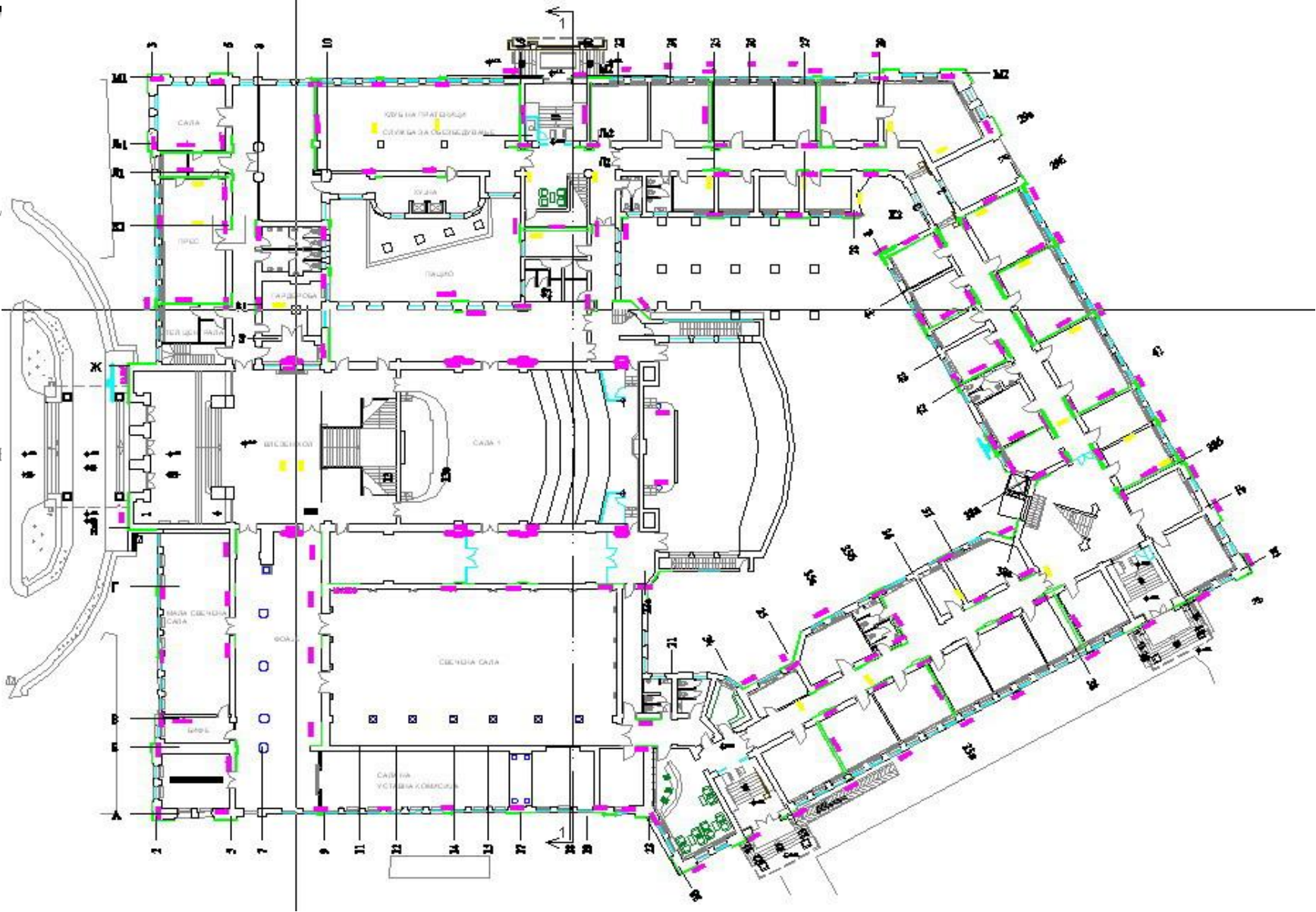
## **additional limitations:**

- requirements as historical building under protection by the Law for cultural heritage
- possibilities for implementation of new RC elements
- avoiding interventions in areas appointed by investor (specific areas, expensive interior with high value or significance...)
- continuous functioning of Parliament during realization of strengthening

## **concept for strengthening:**

- Variant solutions from the aspect of stability, economy and possibility for construction
- Selection of most appropriate solution using classical methods and elements using the same building materials as the existing ones

# Technical Solution for Structural Strengthening



### 3. Analysis of Strengthened State

- modeling: **SW** - separate walls,
- redefined masses
- redefined mechanical characteristics

**INPUT:**

Shear base  $K = 0.24$  (24% G)

**new RC elements:**

MB 30 MPa, RA 400/500

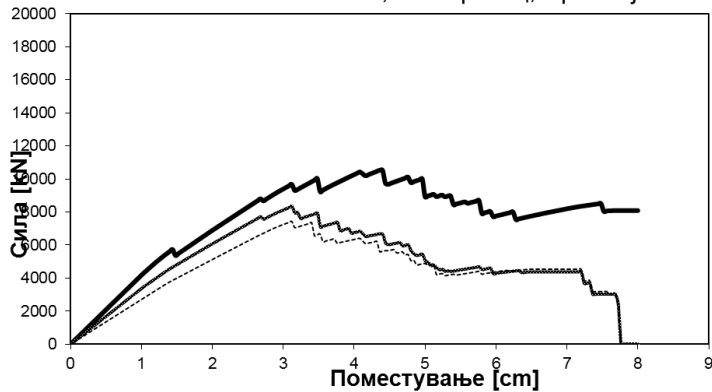
Structural unit	required bearing capacity (% of weight)	Bearing capacity (% of weight)		Required Ductility (max)		Ductility Capacity (max)	
		x-x	y-y	x-x	y-y	x-x	y-y
<b>Separated walls SW</b>							
L1	24	24.8	23.7	2.10	2.40	2.80	2.75
L2		27.8	24.0	1.60	3.70	3.09	4.82
L4		22.1	21.9	2.30	2.10	2.54	2.22
L5		23.1	23.7	1.80	2.20	2.74	2.75
L6		31.2	34.0	1.60	1.10	1.95	2.33
L7		24.5	23.2	2.00	2.60	2.23	2.71

# Effectiveness of Strengthened State

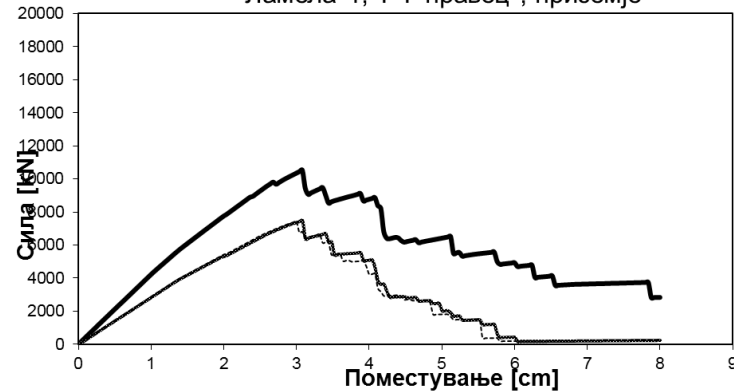
- Comparison of **story diagrams** strength – deformability
- Comparison of **energy dissipation capacity**

--- existing, — another story, — strengthened

Ламела 1, X-X правец, приземје

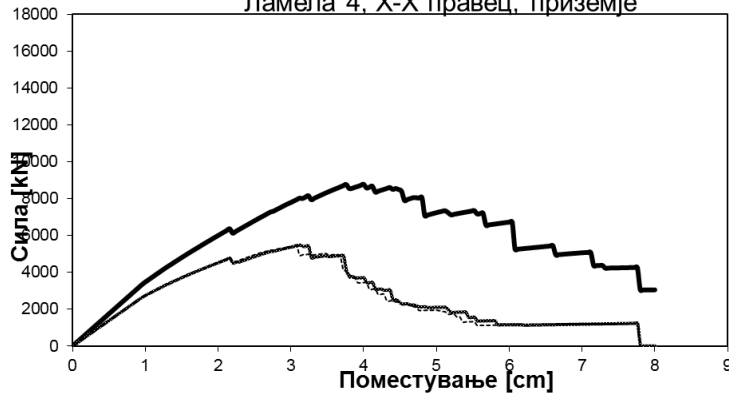


Ламела 1, Y-Y правец, приземје

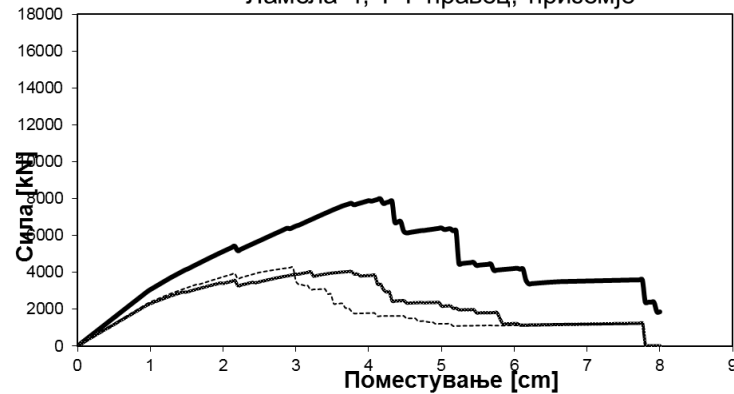


L1

Ламела 4, X-X правец, приземје



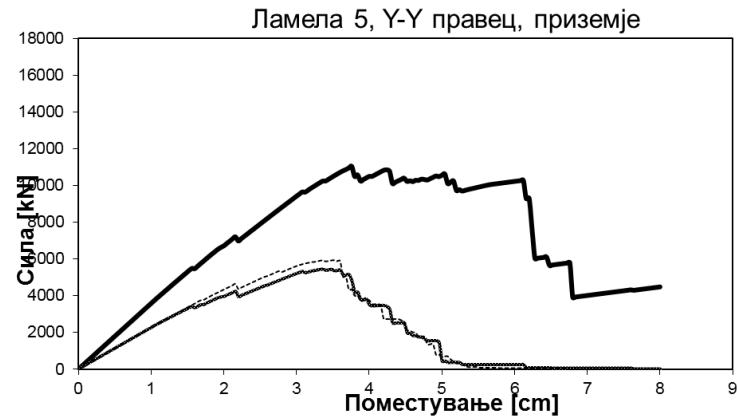
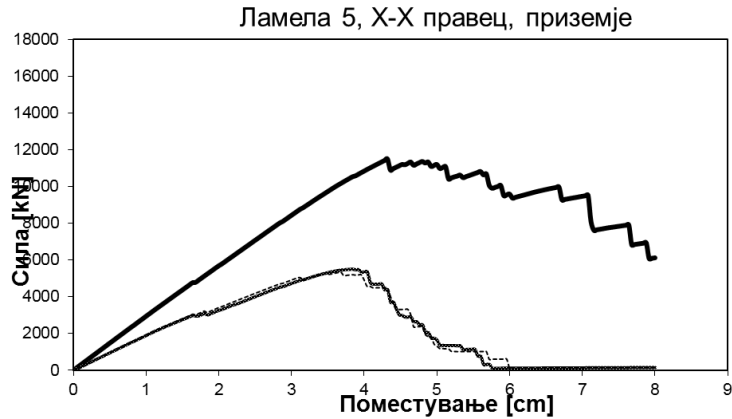
Ламела 4, Y-Y правец, приземје



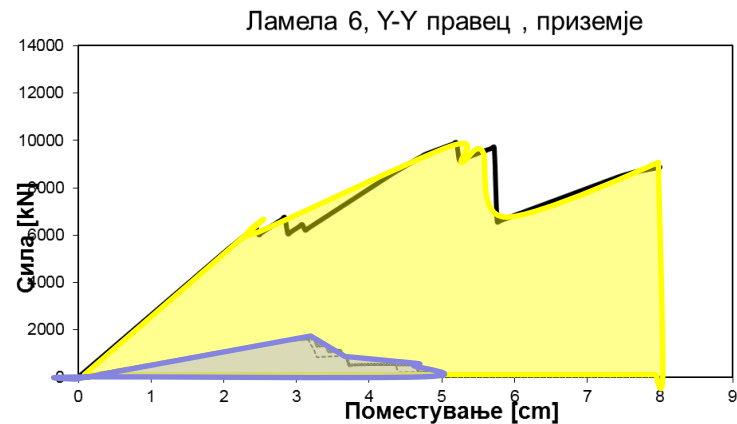
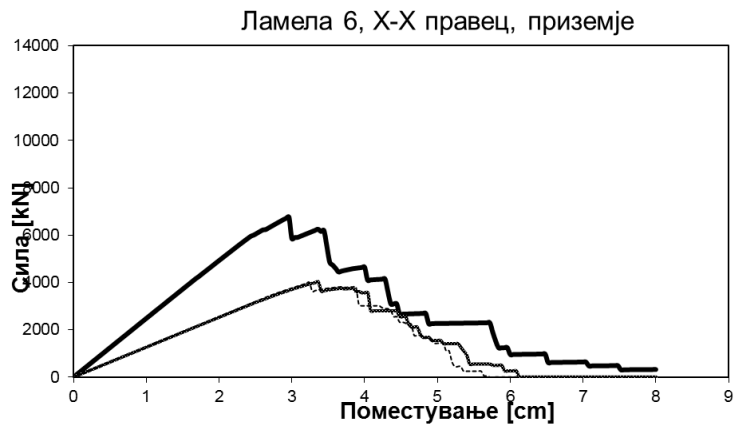
L2

# Effectiveness of Strengthened State

--- existing, — another story, — strengthened



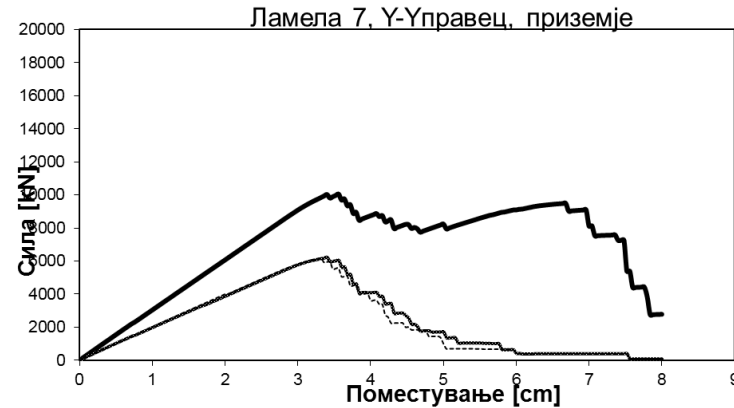
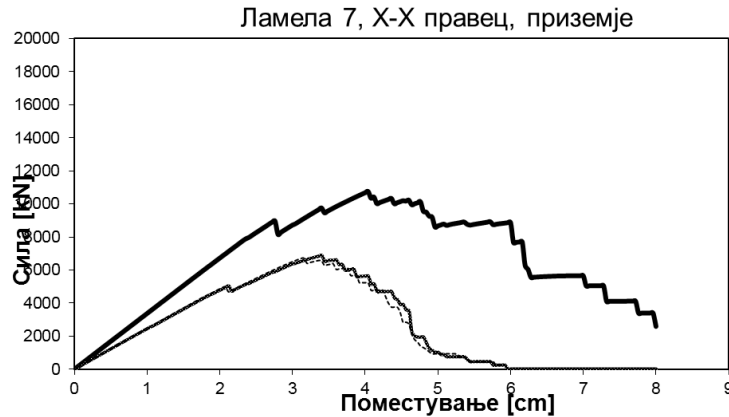
L5



L6

# Effectiveness of Strengthened State

--- existing, — another story, — strengthened



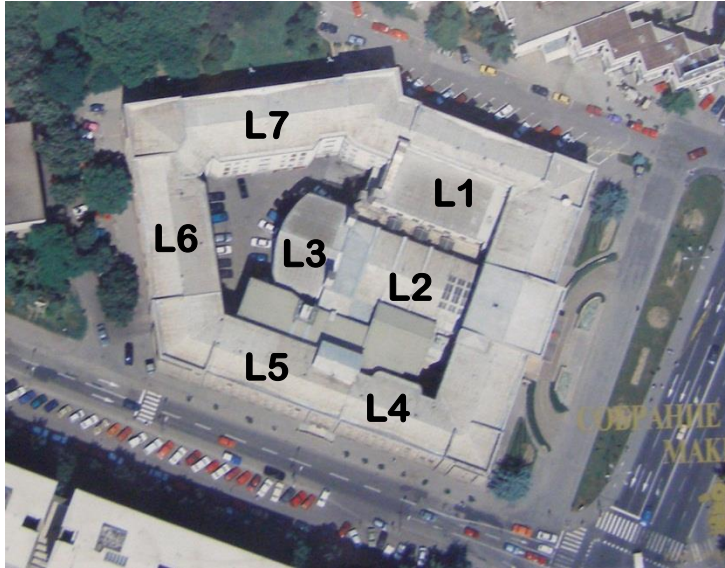
L7

- ✓ Optimization of strength, bearing and deformability capacity
- ✓ Improved structural stability for expected seismic effects



# Realization of the strengthening process

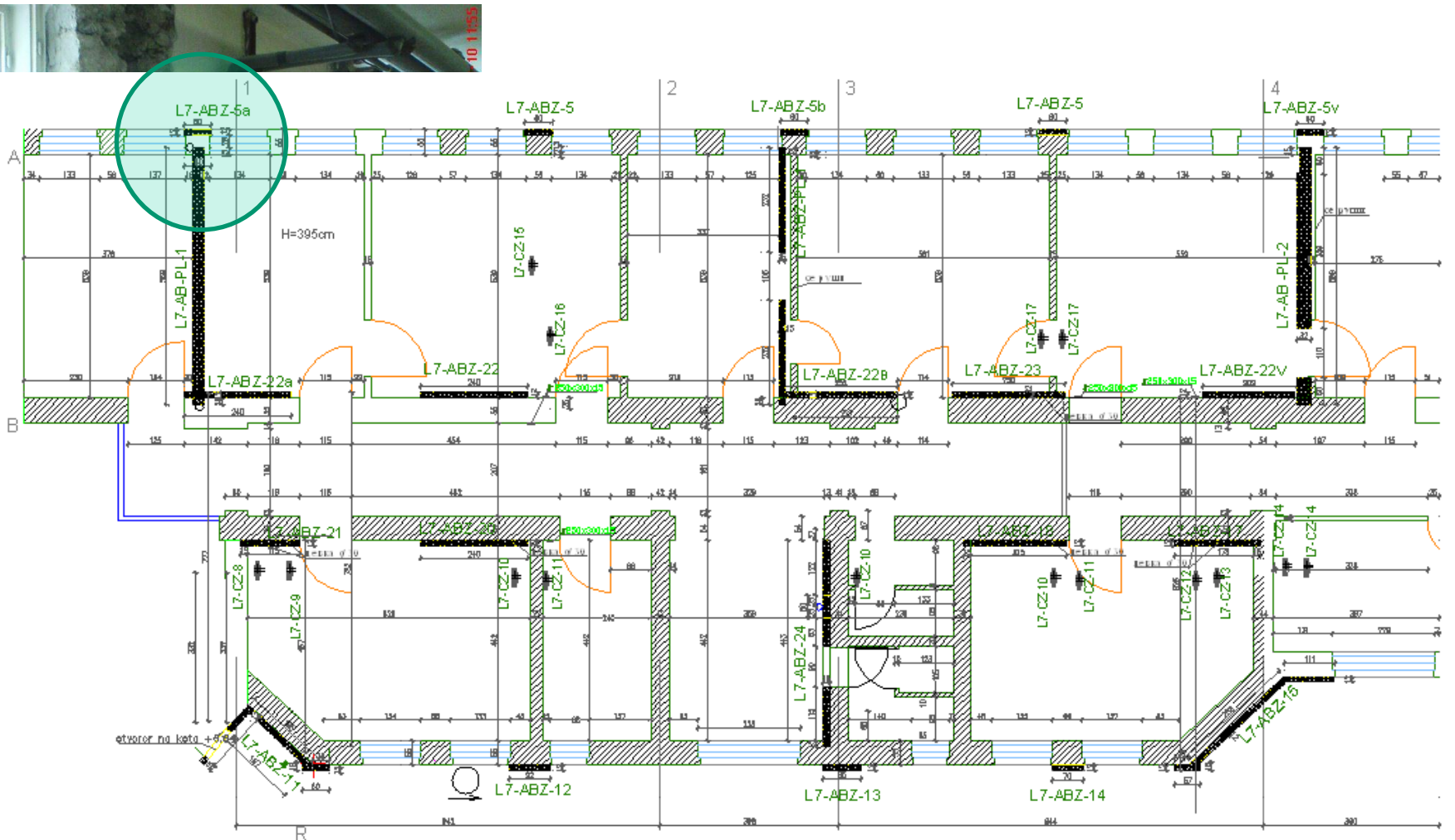
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❑ **L1, L2, L3, L7 , part of L6 finished until September 2011**

- ❑ **Starts in April 2010 with L7**
- ❑ **Continuous functioning of the Parliament**
- ❑ **Continuous supervising by IZIIS' team that encompass:**
  - Detailed inspection of the geometry of individual units
  - Direction of activities and prescribing of order, regime and technology of incorporation of strengthening;
  - Elaboration of variant solutions for modification of individual elements due to newly created and limiting conditions of performance of the works on field;
  - Definition of final solutions based on control computations and engineering knowledge harmonized with the possibilities of the contractor and the conditions for performance of the works;
  - Definition of technical solution for strengthening of new positions that arose from the necessity for structural interventions (L3, L1) after the beginning of the works

# Realization of the strengthening process



# Realization of the strengthening process

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# Realization of the strengthening process

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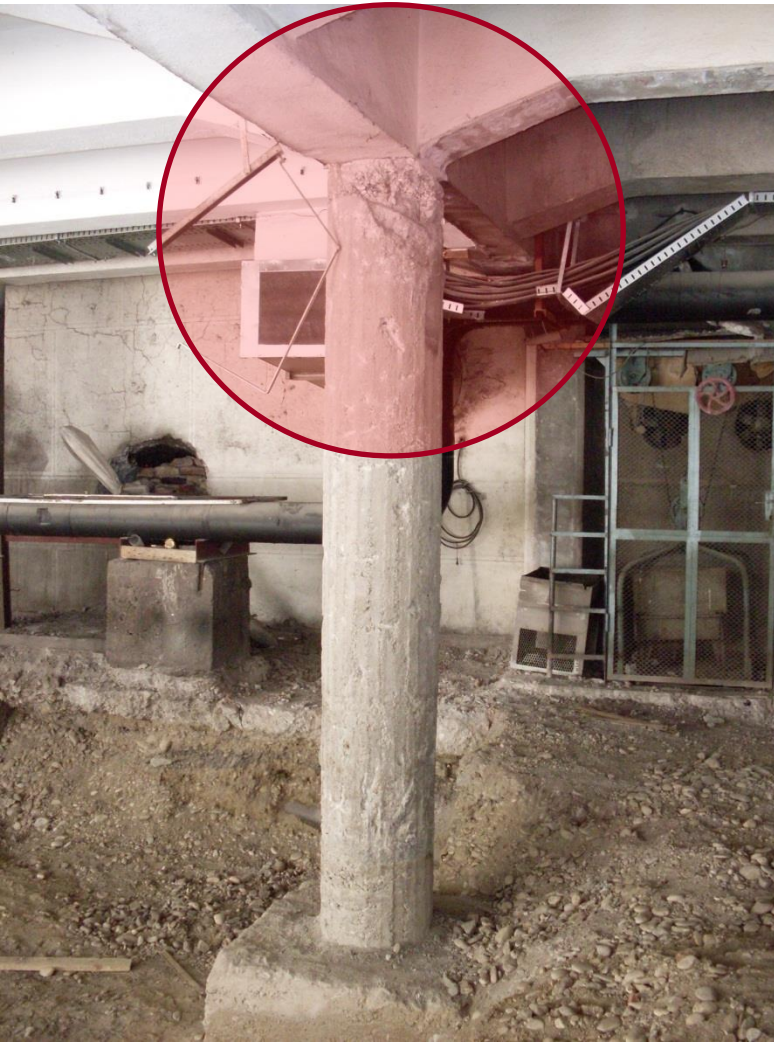


# Realization of the strengthening process

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# Realization of the



# Realization of the strengthening process

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# Realization of the strengthening process





# Realization of the strengthening process



# Realization of the strengthening process



# Realization of the strengthening process



# North Macedonia Parliament (2014)

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**no damage during 2016 earthquake with M 5.3**





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**THANK YOU**

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